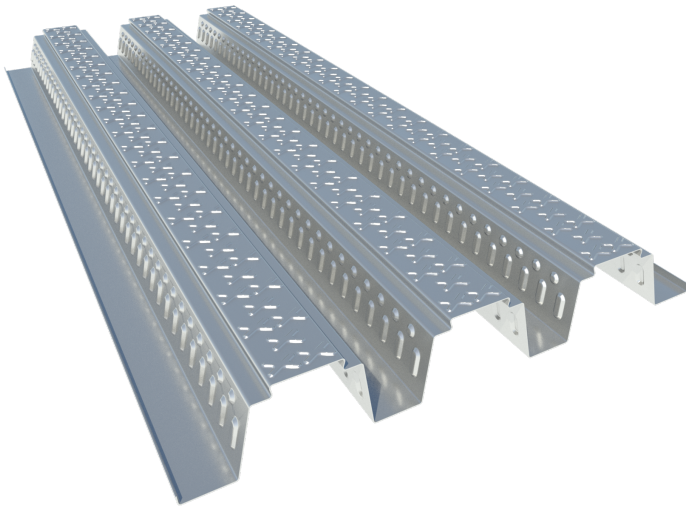


# MT-100 COMPOSITE SLAB

## COMPOSITE SLABS



### RAW MATERIAL

Steel

### THICKNESSES mm (in.)

**0.75 to 1.2**  
 (0.029-0.047)

### FINISH

Galvanized

### USEFUL WIDTH

675 mm (26.57 in.)

### THICKNESS mm (in.)

	0.75 (0.029)	0.80 (0.031)	1.00 (0.039)	1.20 (0.047)
P (kg/m <sup>2</sup> )	10.90	11.63	14.54	17.44
I (cm <sup>4</sup> /m)	182.64	195.78	244.81	294.72
W (cm <sup>3</sup> /m) - upper fiber	31.95	34.50	43.09	52.06
Ap (mm <sup>2</sup> /m)	1297	1385	1732	2078

P=profile weight per square meter I=profile inertia per linear meter W=resistant module profile per linear meter  
 Ap=useful section of steel per line



## DESCRIPTION AND APPLICATION

The composite slab represents the most suitable construction solution for all those constructions where both the maximum technical and mechanical performance are required, as well as quick execution and guarantees. Thanks to its superior characteristics, it adapts to any building type (industrial, commercial, sports, residential). It has significant economic benefits, especially if taken into account at the beginning of the project: it involves a decrease in the average thickness of the slab, and therefore a reduction in weight that translates into a reduction in the resistant section of the structure (pillars, beams, foundations).

The foundation of composite slabs lies in the technology used to enhance the adhesion between the shaped steel sheet and the concrete. This technology is also called composite slab (or what is known as forjado colaborante in Spanish) because of the "collaboration" between the two materials that make up the slab, to cope with the stresses generated by the loads. The mechanical adhesion of the two components is performed through the indentations in the sloping flanks of the galvanized steel profile. Chemical adhesion alone would not be sufficient to ensure efficient bonding that actually works the composite slab as a mixed structure.

The characteristics of the MT-100 composite slab have been developed in collaboration with the Structures Group of the Continuum Mechanics Department of the School of Senior Engineers of Seville (Spain), within a framework of cooperation with AICIA— Andalusian Association for Research and Industrial Cooperation.

The experimental tests carried out comply with the requirements of the Eurocode 4 and Eurocode 3 standards, the only reference standards and mandatory standards at the European level.

The values published in the tables refer to the permissible static overload and the section of reinforcement at the negative bending moment in the case of intermediate supports. Tests to break the slabs of different typology have provided the characteristic parameters "m" and "k" that define the reference line of the MT-100 composite slab. This reference line provides the allowable overload data depending on the thickness of the sheet and the depth of the floor slab.

After obtaining these values, following the testing procedures described in EC4, they have been verified by means of the mandatory verification tests.

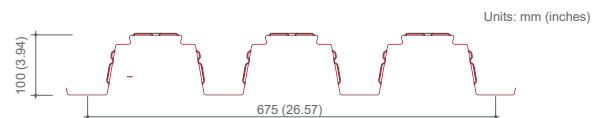
## GEOMETRIC SPECIFICATIONS

## APPLIED STANDARDS

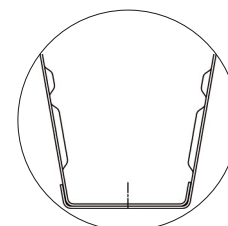
Geometric Specifications			
Characteristic	Value	Units	Tolerance / Standard
Profile thickness (h)	100 (3.94)	mm (in.)	±1.5 EN 1090
Wave pitch	225	mm	+4/-1 EN 1090
Width of the ridge and valley	132.5/65	mm	+4/-1 EN 1090
Useful width (w)	675 (26.57)	mm (in.)	(±0.1* h) and ≤15 EN 1090
Protruding surface of core depth	3.5	mm	-0.5/+1 EN 1090
Length [L]	1600 (62.99) to 14,000 (551.18)	mm (in.)	+20/-5 EN 1090
Height / Width stiffener	15/88	mm	-0.5 to +1/ +0.1 EN 1090
Execution class	EXC2		EN 1090

Ref. Standard	Description
EN 508-1	Products for sheet metal roofing and cladding. Specify for self-supporting steel sheet products. Part 1: steel.
EN 10143	Sheets and strips of steel with continuous metal coating by hot dipping. Dimensional and shape tolerances.
EN 10346	Flat steel products, continuous coated by hot dipping. Technical supply conditions.
EN 1090-2	Completion of steel and aluminum structures. Part 2: Technical requirements for steel structures.
EN 1090-4	Completion of steel and aluminum structures. Part 4: Technical requirements for cold-formed structural elements and steel structures for roof, ceiling, flooring and wall applications.

Features of the Profile			
Characteristic	Value	Units	Tolerance / Standard
Deviation from straightness	≤ to the tolerance	mm	±2/mL (max.10) EN 1090
Deviation from quadrature	≤ to the tolerance	mm	≤ 0.005 w EN 1090
Deviation of the side overlap	≤ to the tolerance	mm	±2 s/500 mm EN 1090
Sheet thickness	0.75 (0.029) to 1.2 (0.047)	mm (in.)	EN 10143
Type of steel	S220GD to S350GD		EN 10346
Emission of cadmium and compounds	COMPLIES - No emissions		EN 1090
Radioactivity emission	COMPLIES - No emissions		EN 1090
Behavior against fire	Broof (t1)		RD 110/2008
Durability	Hot-dip galvanized		EN 10346
Fire resistance	Class A1		EN 13501-1
Load bearing capacity	See load tables		EN 1993 - EC3 and EC



SECTION PROFILE



OVERLAP DETAIL





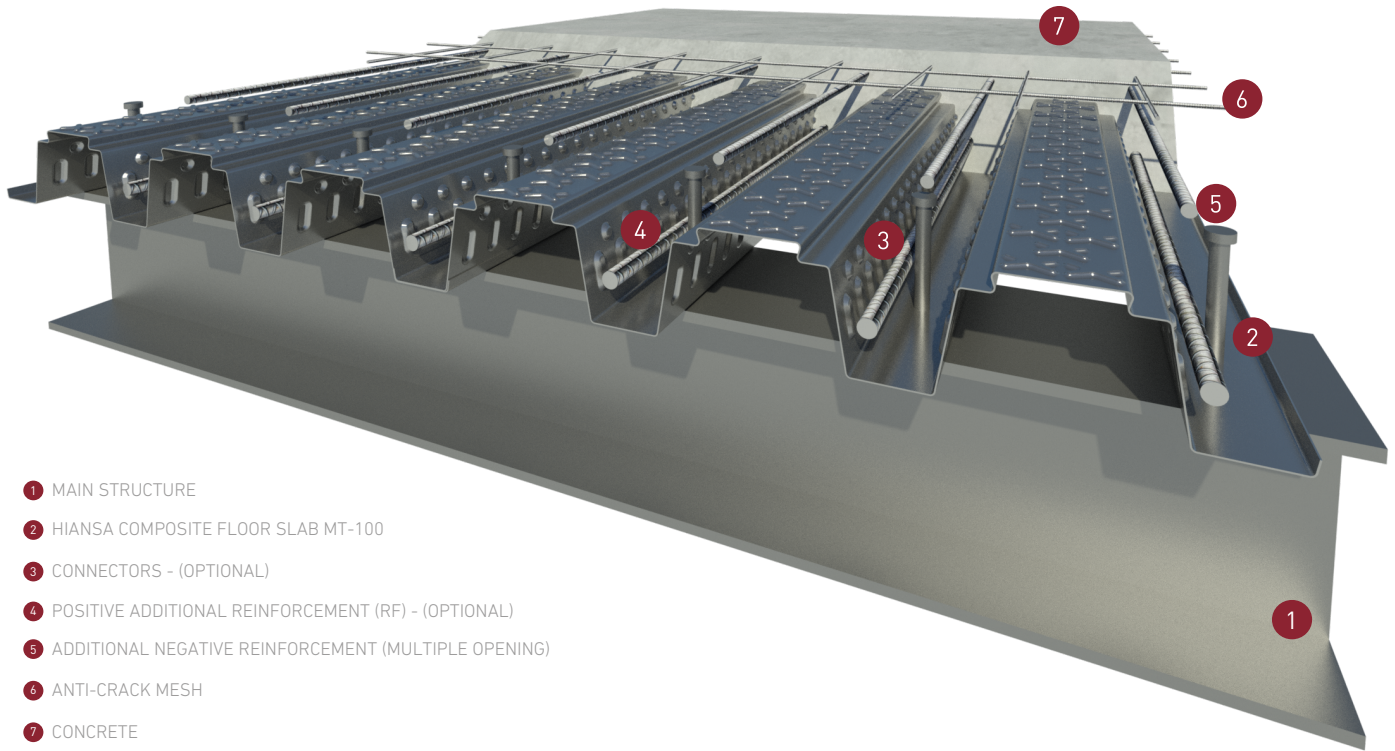




# MT-100 COMPOSITE SLAB

## COMPOSITE SLABS

### TYPE DETAILS



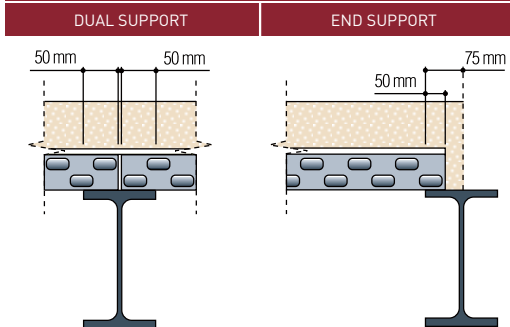
- ① MAIN STRUCTURE
- ② HIANSA COMPOSITE FLOOR SLAB MT-100
- ③ CONNECTORS - (OPTIONAL)
- ④ POSITIVE ADDITIONAL REINFORCEMENT (RF) - (OPTIONAL)
- ⑤ ADDITIONAL NEGATIVE REINFORCEMENT (MULTIPLE OPENING)
- ⑥ ANTI-CRACK MESH
- ⑦ CONCRETE

Illustrative view

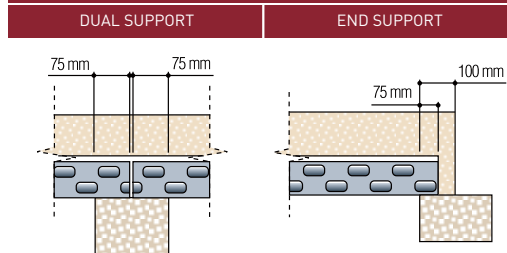
Section type MT-100 composite slab, in which all the reinforcements that can be placed according to the calculation requirements set by the Designer are indicated. Even connectors that are welded or bolted will be necessary when the slab is required to work integrally with the supporting metal beam.

### CONDITION OF SUPPORT OF THE SHEETS ON BEAMS

#### SUPPORT ON STEEL AND CONCRETE

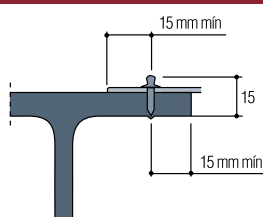


#### SUPPORT ON OTHER MATERIALS (BRICK OR BLOCK)

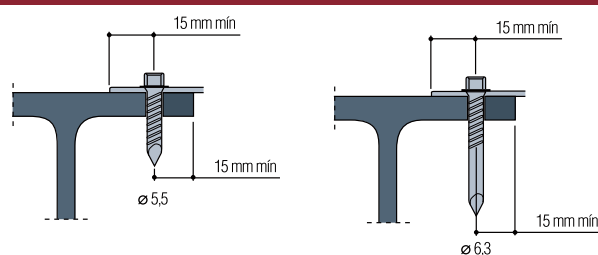


The attachment of the slab to the structure will be by screw, nail or welding, at the discretion of the Designer and always respecting the minimum measurements indicated for each case in the attached figures. It is recommended to attach each sheet one at a time and check at the end that they are all secured.

#### FASTENING BY GUN



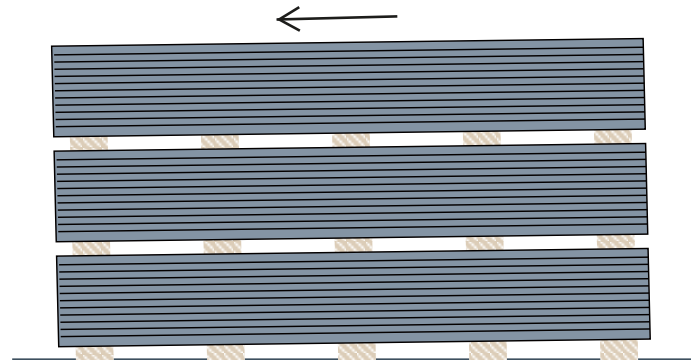
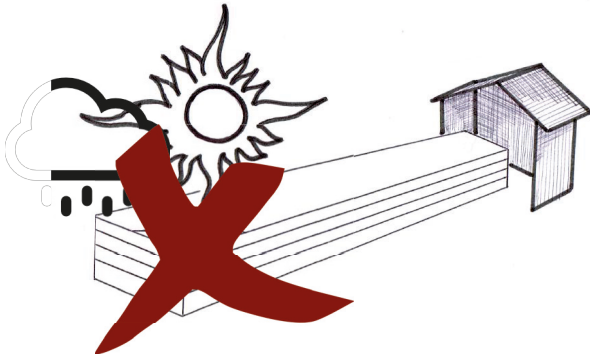
#### FASTENING BY SCREW



## STOCKING OF MATERIAL

In order to avoid the action of wind, humidity, condensation and rain, it is recommended that the galvanized steel material be stored in covered, ventilated areas and in an atmosphere that is as dry as possible.

In case of outdoor storage, the packages must be isolated from the ground by means of blocks of different height in order to obtain a slope that favors the evacuation of the water. They must also be covered with tarpaulins or plastics ensuring proper ventilation to avoid the concentration of water or excessive humidity that can cause white oxide to appear that only affects the aesthetics of the material without reducing its resistant properties.



Elevation view

## STRUTTING THE SLAB

Strutting the slab is understood as the placement of intermediate supports to temporarily reduce the distance between supports during the pouring and setting phases of concrete. Once the sheets have been secured, where necessary, a strut will be placed in the middle of the section. In case of needing two struts (large section of free span) the struts will be placed at  $1/3$  and  $2/3$  of the free span of the section. The sketch illustrates the correct way to place a strut.

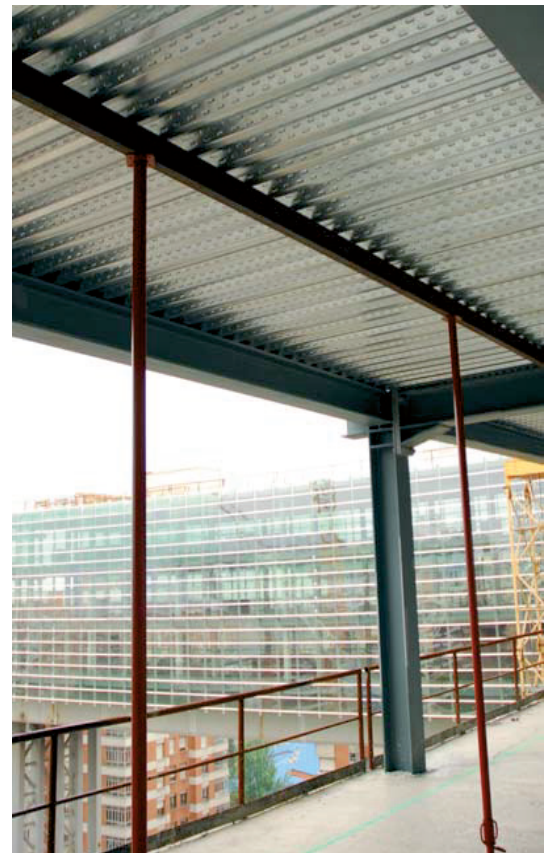
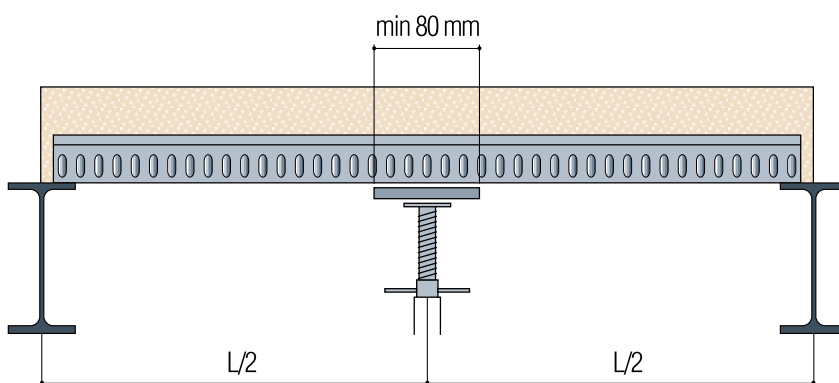
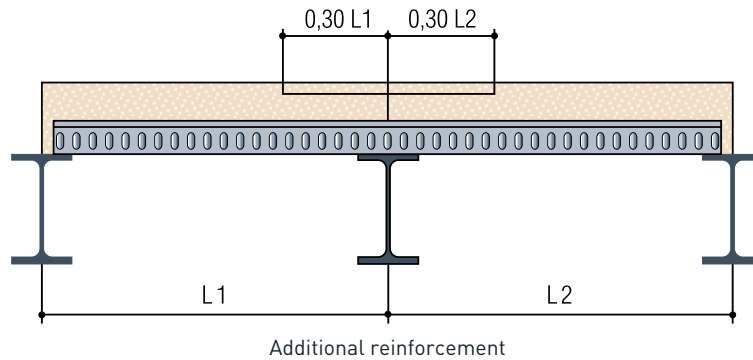


Image of temporary strutting

## NEGATIVE REINFORCEMENT

When the designed slab is continuous, that is, it has intermediate supports, negative bending moments occur on these. It is then necessary to place this type of reinforcement, at a depth of 25 mm with respect to the upper face of the slab. The corrugated bars must be of sufficient length to cover one third of the span of each of the adjacent openings, as shown in the attached sketch. The minimum section of reinforcement required withstand these negative bending moments is detailed in the corresponding calculations.



## ANTI-CRACK MESH

Its main mission is to cope with the retraction efforts generated by the drying of concrete, avoiding its cracking. It also contributes to the distribution of small point loads acting on the slab. It must be placed at a depth of 20 mm with respect to the upper face of the slab, covering its entire surface.

**ANTI-CRACKING MESH IN SLAB COMPRESSION LAYER (mm)**

		H (cm)																
		10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	
Mesh	MT-100	200x200x4	-	-	-	-	✓	✓	✓	-	-	-	-	-	-	-	-	
		200x200x5	-	-	-	-	-	-	-	✓	✓	✓	✓	✓	✓	-	-	
		200x200x6	-	-	-	-	-	-	-	-	-	-	-	-	-	-	✓	✓

## OWN WEIGHT AND VOLUME OF CONCRETE

**VALUES OF OWN WEIGHT OF THE COMPOSITE SLAB [kN/ m2]**

		H (cm)																
		10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	
Profile	MT-100 e=0.75 mm	-	-	-	-	1.9	2.14	2.38	2.62	2.86	3.1	3.34	3.58	3.82	4.07	4.3	4.54	
	MT-100 e=0.8 mm	-	-	-	-	1.91	2.15	2.39	2.63	2.87	3.11	3.35	3.59	3.83	4.07	4.31	4.55	
	MT-100 e=1.0 mm	-	-	-	-	1.93	2.17	2.41	2.65	2.89	3.13	3.37	3.61	3.85	4.09	4.33	4.57	
	MT-100 e=1.2 mm	-	-	-	-	1.96	2.2	2.44	2.68	2.92	3.16	3.4	3.64	3.88	4.12	4.36	4.6	

**VOLUME OF CONCRETE PER UNIT AREA [m3/ m2]**

		H (cm)																
		10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	
Profile	MT-100 e=0.75 mm	-	-	-	-	0.075	0.085	0.095	0.105	0.115	0.125	0.135	0.145	0.155	0.165	0.175	0.185	
	MT-100 e=0.8 mm	-	-	-	-	0.075	0.085	0.095	0.105	0.115	0.125	0.135	0.145	0.155	0.165	0.175	0.185	
	MT-100 e=1.0 mm	-	-	-	-	0.075	0.085	0.095	0.105	0.115	0.125	0.135	0.145	0.155	0.165	0.175	0.185	
	MT-100 e=1.2 mm	-	-	-	-	0.075	0.085	0.095	0.105	0.115	0.125	0.135	0.145	0.155	0.165	0.175	0.185	



## BEHAVIOR AGAINST FIRE

The R factor is the load bearing capacity of a composite slab in a fire situation. According to Eurocode 4 Part 1.2, for this type of solution it will be 30 minutes (R-30). This data does not need any verification, as long as the calculation of the composite slab has been made in accordance with the specifications of Eurocode 4 Part 1.1.

If the project requires a fire resistance of more than 30 minutes (R-30), the designer can opt for different solutions:

- Incorporate a fire protection system on the underside of the slab. One option is to create a continuous coating of homogeneous thickness with mortars or paints or incorporate false ceilings of plasterboard or other materials (taking special care to ensure watertightness of the joints between elements).
- Incorporate traction reinforcements into the slab. This increases the bearing capacity of the slab in a fire situation (criterion R), but not the thermal insulation capacity (I). The thermal insulation capacity continues to depend on the effective thickness of the slab and the additional protection provided by the underside of the steel profile (\*).

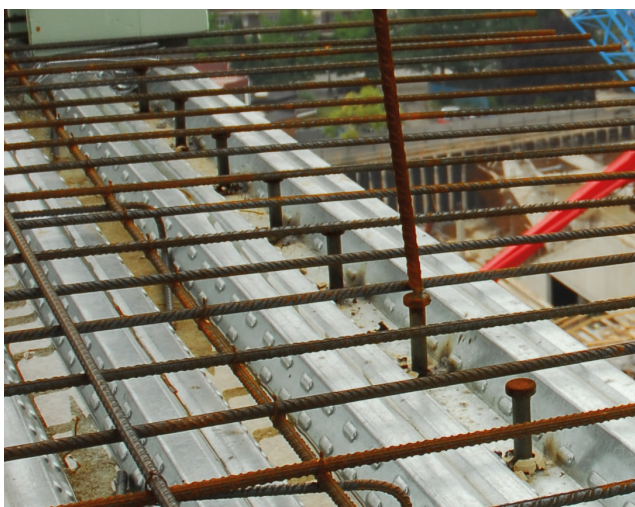
**(\*) Contact our Technical Department for cases in which an R factor greater than 30 minutes is required, in order to evaluate the most optimal solution in each case and receive personalized advice.**

## CONNECTORS - MIXED BEAM SOLUTION CONNECTED TO STRUCTURE

It is important to note that the connectors do not influence the actual strength of the composite slab. That is, the fact of having connectors, does not make the slab more resistant than when they are not used.

In this constructive solution, the profile for composite slab is attached to the metal structure by means of the connectors. The slab becomes part of the same load-bearing structure of the building, ceasing to be a monolithic element whose weight is supported by the beams and pillars on which it rests. It functions as a compression layer of the resistant section, which in this way sees its strength properties noticeably increased. This allows considering in the calculations, the sum of the resistant sections of the metal beam and the slab. The decision about the type of structure to be adopted and the corresponding calculation are the responsibility of the Designer.

These connectors may be welded through the slab sheet to the support structure or mechanically fixed by pneumatic gun and flashing or the like.



Soldered connectors



Mechanically fixed connectors

## RECOMMENDATIONS FOR ASSEMBLY

### Pouring of the concrete:

The concreting on the corrugated sheets will be carried out by traditional methods: pumps and pipes or cupola. Any oil, dirt, remaining greasiness from the manufacturing process or harmful substance, present on the upper face of the profile, must be removed before starting the pouring phase of the concrete. To achieve the final properties of the slab specified in the project, utmost care must be taken in this phase, avoiding excessive deformation of the slab, segregation of the aggregate or loss of slurry. The concrete will be poured as far as possible on the support beams of the slab, from the minimum possible height. It is necessary to use a concrete outlet pipe equipped with a handle that allows easy and practical handling, since in no case will the concrete be poured from a height greater than 30 cm. It is necessary to avoid any accumulation of material, and distribute it longitudinally to the ribs of the steel profile, from the beams to the spans. The forklift circulation will be carried out on 30 mm thick planks placed on the mesh, making sure that no more than three operators coincide in the same area of the slab at the same time.

To ensure the proper functioning of the slab, a satisfactory compaction must be made around the connectors, the reinforcements and on the relief of the sheet. It is not necessary to vibrate the concrete. In case of grout losses with the consequent appearance of stains on the lower part of the profile, it is advisable to clean with a simple stream of water before the drying.

### Opening of holes in the slabs:

Generally, in the works it is necessary to provide apertures for the installations to pass through and downspouts through the slab. In this case, the apertures must be made prior to concreting, using expanded polystyrene blocks or any other type of formwork. When the side of the aperture is larger than a wave, it will be necessary to reinforce the perimeter of the aperture longitudinally and transversely at the structural level.

In general, it can be said that:

- Apertures up to 300 mm on the side do not require reinforcement.
- Apertures with sides between 300 and 700 mm in length require reinforcement bars.
- The apertures with a side greater than 700 mm in length require the placement of auxiliary support structures. To make these apertures, the metal profile will be cut as long as the concrete is cured. It is important not to drill the slab with percussive equipment once it is set, since vibrations can affect the collaboration between the steel sheet and the concrete, generating a loss of adhesion and therefore of load bearing capacity.

### Type of trims:

To expedite the construction of a composite slab and optimize the completion time, Hiansa S.A. has created exclusive galvanized steel finishes. These are pieces that, even without being essential, are very useful, since they replace certain formwork operations that would otherwise be done in a more artisanal way and approximatively in the construction:

- Trim for edge of slab (R1).
- Trim for brace (R2).
- Trim for shifting direction of slab (R3)



Trim for edge of  
slabs (R1) – LINEAR



Trim for brace  
(R2) - SINGLE-POINT



Trim for shifting  
of slab  
(R3) – LINEAR

## CALCULATION CONSIDERATIONS

### Calculation hypotheses:

The results contained in the static overload tables, obtained according to the procedure established by the EC4 and EC3 Regulations, are based on the following calculation hypotheses:

- The loads acting on the slab are distributed and predominantly static.
- The spans of the slab are located in the direction of the ribs of the sheet.
- For the study of the slabs in the service phase, elastic analysis is used; for the flexural strength test, plastic theory is considered.
- The results of the tables refer to a composite slab without connectors; that is, they do not describe the behavior of the mixed beam solution.
- The concrete considered in the calculation is an HA-25 (\*).
- The yield strength considered in the calculation of the steel of the MT-100 profile is 220 MPa (\*), and the partial safety coefficient for Ultimate Limit States of the steel of the profile is 1.10.
- The calculation model used considers the following limit states: in the execution phase, bending represents the ultimate limit state, and deformation represents the service limit state. In the service phase, the ultimate limit states are represented by the bending, the shear forces, the vertical shears, while the ultimate limit state is the deformation.
- Deflection criterion when the ribbed steel sheet acts as a formwork:  $f \leftarrow l/250$  or  $f \leftarrow 20$  mm(\*), with L= clear span between supports. In the calculation of these deformations, the weight of the sheet and fresh concrete are considered, but the setting loads are not considered, since they are temporary.
- Deflection criterion in the service phase:  $f \leftarrow l/250$  (\*) in any case referred to in the tables.
- Coefficients of increase of the loads used in the calculations:
  - Coefficient of increase of own weight: 1.35.
  - Coefficient of increase of permanent loads: 1.35.
  - Coefficient of increase of use loads: 1.50.
- The values of the "Service Load Tables for Profile MT-100" have been calculated in accordance with the specifications of EC4 part 1.1 in the construction phase of the slab, and as a mixed slab in the service phase thereof. The tables refer to a generic type of slab defined in the previous points. The project calculator is responsible for calculating the floor slab according to the specifics of the loads involved, the materials used and others specific to each project. The static overload values contained in the tables are the maximum allowable overload values in service, where the loads represent the sum of the permanent loads and the overloads of use acting on the slab. The weight of the composite slab itself has already been taken into account in the calculations.

*[\*] For other values, contact the Technical Department to evaluate the most optimal solution in each case and receive personalized advice.*

## CALCULATION CONSIDERATIONS

**Interpretation of the different shadings in the allowable overload tables: diversity of theoretical approach by insertion of the strut (during the setting of the slab).**

The user of the overload tables of the composite slab with MT-100 profile might be surprised to see how, at a certain point, by increasing the thickness of the concrete slab by 1 cm, the allowable overload drops significantly. This jump in the values corresponds to entering into the shoring area, shaded in pink of the tables. This is due to the different theoretical approach that sustains the study and verification of a structure that is detached and that of an attached structure (as set out in Eurocode 4 and Eurocode 3). An unsupported steel sheet, in the phase of setting the floor slab, is deformed proportionally to the weight of the concrete poured.

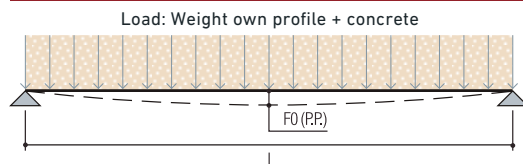
Once set, the slab presents sagging ( $f_0$ ) and the sheet has an internal tension corresponding to its deformation.

When this slab is loaded with weight (load  $Q$  evenly distributed), the maximum value of bending moment (corresponding to load  $Q$ ) will be recorded in the center of the span. It is time to check the slab at the various stresses present (bending moment, shear stress, bond stress): in almost all cases the slab will break by reaching the maximum bond stress moment. It is fair to say that the load that has determined the slippage between the concrete and the steel sheet is equal to the sum of the slab's own weight and the applied load  $Q$ .

In the detached structures, the intermediate strut divides the free span between supports in two, and the sag ( $f_0'$ ) that is registered is significantly lower than the sag  $f_0$  (registered by the same unshored slab). By approximation it can be said that the sag  $f_0'$  is equal to 0. During the setting of the concrete, the sheet does not present tension, it being the strut that supports the weight of the poured concrete. Once the concrete has been set, removing the strut and applying a  $Q$  load to the structure, the slab is checked for all the stresses present. Once again the collapse occurs by reaching the Ultimate Limit State at bond stress: in this case, the load  $Q$  determines the breakage of the slab. In the tables of admissible overload it is not permissible to add the weight of the slab itself to the value recorded during the test to break the slab.

In summary, in a detached structure, it is permissible to add the own weight of the slab to the overload value of registered use, because the structure had already assumed this load (the own weight) before setting: the sag  $f_0$  represents the deformation corresponding to the internal stress of the sheet generated by the pouring of the concrete.

### UNSHORED CONSTRUCTION, UNHARDENED STATE



Internal stress to the steel sheet:  $\sigma_0 = \frac{PP \cdot L^2}{8 W_{xx}}$   $f_0(PP) \gg f_1(PP)$

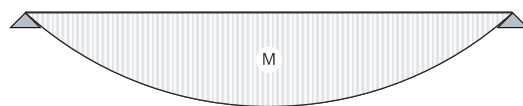
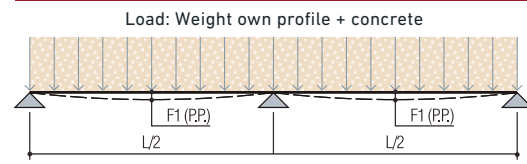
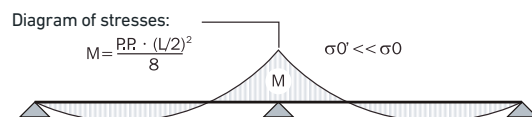


Diagram of stresses:  $M = \frac{PP \cdot L^2}{8}$   $\sigma_0 \gg \sigma_0'$  the sheet is pre-stressed during the setting of the concrete, presenting sag ( $f_0$ )

### SHORED CONSTRUCTION, UNHARDENED STATE



Internal stress to the steel sheet:  $\sigma_0' = \frac{PP \cdot (L/2)^2}{8 W_{xx}}$   $f_1(PP) \ll f_0(PP)$   
 $f_1(PP) = 0$  (by approximation)



### UNSHORED CONSTRUCTION, HARDENED STATE

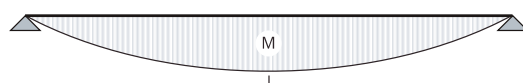
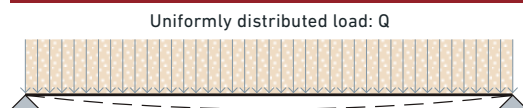


Diagram of stresses:  $M = \frac{Q \cdot L^2}{8}$  Breaking point tests for Ultimate Limit State:  $Q_u$  bending moment /  $Q_u$  shear stress /  $Q_u$  bond stress

### SHORED CONSTRUCTION, HARDENED STATE

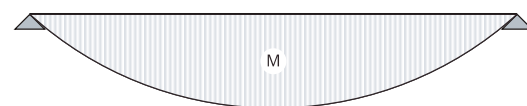
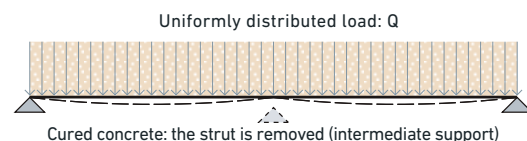


Diagram of stresses:  $M = \frac{Q_{design} \cdot L^2}{8}$   $Q_{design} = (PP + Q \text{ load})$  Breaking point tests for Ultimate Limit State:  $Q_u$  bending moment /  $Q_u$  shear stress /  $Q_u$  bond stress